#### GENERAL NOTES:

- 1. ALL DIMENSIONS ARE IN mm AND LEVELS IN mPD.
- 2. ALL DESIGN SHALL COMPLY WITH HONG KONG BUILDING (CONSTRUCTION) REGULATION 1990 EDITION AND STRUCTURAL DESIGN OF STEEL IS IN ACCORDANCE WITH THE CODE OF PRACTICE FOR THE STRUCTURAL USE OF STEEL 2011.
- THIS SET OF DRAWINGS SHALL BE READ IN CONJUNCTION WITH THE FOUNDATION PLAN. THE CONTRACTOR SHALL CHECK ALL RELEVANT DRAWINGS AND VERIFY LEVELS AND DIMENSIONS IN ADVANCE OF THE WORK AND WORK AND REPORT ANY DISCREPANCY TO THE ENGINEER IMMEDIATELY.
- . ALL EXCAVATION SHALL BE BACKFILLED TO THE PROPOSED GROUND LEVEL AFTER COMPLETION OF FOUNDATION CONSTRUCTION.
- THE CONSTRUCTION SEQUENCE FOR EXCAVATION AND LATERAL SUPPORT, REFER TO DRG. NO. S-ELS-006 TO 007.
- THE INSTALLATION OF SHEET PILE SHALL BE WALL CARRIED OUT TO ACCORDING TO APPROVAL DRAWINGS PRIOR TO THE COMMENCEMENT OF EXCAVATION AND LATERAL SUPPORT WORKS.

#### NOTES ON CONSTRUCTION MATERIAL

- 1. STRUCTURAL STEEL MEMBERS a. ALL STRUCTURAL STEEL MEMBERS SHALL BE GRADE S355 (CLASS 1) WELDABLE
- STRUCTURAL STEEL AND COMPLY WITH TO BS EN 10025:2004. b. ALL WELDING SHALL COMPLY WITH THE CODE OF PRACTICE FOR STRUCTURAL USE OF
- STEEL 2005, BS EN 1011-1:2009, BS EN 1011-2:2001 & BS EN 499:1995. c. ALL CONNECTIONS SHALL BE 10mm FILLET WELDS ALL ROUNDED UNLESS OTHERWISE
- d. SAMPLES OF WELDING MATERIALS USED SHALL BE TESTED & TEST RESULTS SHALL BE SUBMITTED TO RSE FOR APPROVAL. ALL WORKS, MATERIALS AND TESTING SUCH AS TESTING OF STEEL BAR SHALL COMPLY WITH GENERAL SPECIFICATION FOR CIVIL ENGINEER WORKS 1992 EDITION AND HONG KONG BUILDING(CONSTRUCTION) REGULATION 1990 EDITION UNLESS OTHERWISE STATED IN THE DRAWING.

#### NOTES FOR EXCAVATION AND LATERAL SUPPORT (ELS) WORKS (TEMPORARY)

- I. THE CONTRACTOR SHALL TAKE FULL RESPONSIBILITY FOR THE ERECTION, MAINTENANCE AND
- REMOVAL OF ALL TEMPORARY WORKS DURING CONSTRUCTION.
- NECESSARY PRECAUTIONS SHALL BE TAKEN TO PREVENT DAMAGE TO EXISTING FOUNDATIONS. DRAINS, PAVEMENTS, FEATURES, SERVICES ETC. SHOULD ANY DAMAGE OCCUR, NOTIFY THE ARCHITECT AND RELEVANT AUTHORITIES CONCERNED IMMEDIATELY AND MAKE GOOD BY THE CONTRACTOR AT NO EXTRA COST AND NO EXTENSION OF TIME.
- ALL TEMPORARY WORKS SHALL BE WITHIN THE SITE BOUNDARY 4. DURING SUBSTRUCTURE CONSTRUCTION, THE GROUNDWATER LEVEL SHALL BE KEPT BELOW THE
- 5. THE CONTRACTOR SHALL INCREASE THE FREQUENCY OF MONITORING AS INSTRUCTED BY THE ENGINEER SHOULD ANY UNDUE GROUND MOVEMENT BE OBSERVED.
- 6. MAX. ANGLE FOR TEMPORARY SOIL CUT SLOPE SHALL BE REFERRED TO PLANS AND SECTIONS. BUT IN NO CIRCUMSTANCE BE GREATER THAN 20?IN MD LAYER.

#### NOTES ON STRUCTURAL STEELWORK

- ALL STRUCTURAL STEELWORK SHALL BE COMPLED WITH CODE OF PRACTICE FOR THE STRUCTURAL USE OF STEEL 2011.
- 2. ALL LEVEL SHOWN ARE IN METERS AND OTHER DIMENSIONS SHOWN ARE IN MILLIMETERS UNLESS OTHERWISE STATED.
- 3. ALL STRUCTURAL STEEL SECTION SHALL BE WELDABLE STRUCTURAL STEEL TO BS EN 10025:2004 UNLESS OTHERWISE NOTED.
- 4. DESIGN SURCHARGE:
- a) BACK SERVICE LANE (2.0m WIDE): 10kPa
- b) RECLAMATION STREET (9.0m WIDE): 20kPa
- c) FOOTPATH ALONG RECLAMATION STREET (2.0m WIDE): 5kPa d) BEARING PRESSURE AT HOARDING FOOTPATH (0.45m WIDE) : 20kPa
- e) D.L. & L.L. OF EXISTING BUILDING VIA PILING SYSTEM
- (REFER TO RECORD PLAN) DATUM FOR SURCHARGE AT
- 2/3 OF THE LENGTH OF PILE MEASURED FROM GROUND LEVEL f) LIVE LOAD FOR EACH LAYER OF WALING/ STRUT : 2kPa

# NOTES ON WELDING

- THE CONTRACTOR SHALL SUBMIT TO AP/ RSE HIS PROPOSED PROCEDURE FOR WELDING, WELDING PROCEDURE WILL BE TESTED IN ACCORDANCE WITH
- BS FN ISO 15614-1:2004+A1:2008. THE CONTRACTOR SHALL ONLY USE QUALIFIED WELDERS WHO HAVE DEMONSTRATED THEIR COMPETENCE IN WELDING TO THE AGREED PROCEDURE. EACH WELDER
- WILL BE TESTED AS DESCRIBED IN BS EN 287-1:2004. 3. ALL WELDS SHALL MEET THE ACCEPTANCE CRITERIA LAID DOWN IN BS EN 1011, 1:2009 & BS EN 1011-2:2001.
- 4. UPON REQUESTED BY THE ARCHITECT WELDS WILL BE TESTED BY RADIOGRÁPHIC EXAMINATION TO BS EN 1435:1997 OR ULTRASONIC EXAMINATION TO BS EN 1714:1998
- 5. UNLESS OTHERWISE APPROVED, ALL SPLICES TO BE CONTINUOUS FULL-STRENGTH FULL PENETRATION BUTT WELDS.
- 6. UNLESS OTHERWISE STATED, ALL FILLET WELDS SHALL BE 8mm ALL/ROUND.
- 7. ALL IMPROPER MATERIALS (e.g. SLAG, DIRT, IRREGULARITIES, OIL étc.) TO BE
- REMOVED FROM JOINTS PRIOR TO WELDING. 8. ALL WELDING SHALL COMPLY WITH BS EN 1011, P.T. 1:2009, P.T. 2:2001.
- 9. SAMPLES OF ALL MATERIALS USED SHALL BE TESTED & TEST RESULTS SHALL BE SUBMITTED TO RSE FOR APPROVAL. ALL WORKS, MATERIALS AND TESTING SUCH AS TESTING OF STEEL BAR SHALL COMPLY WITH GENERAL SPECIFICATION FOR CIVIL ENGINEER WORKS 1992 EDITION AND HONG KONG BUILDING (CONSTRUCTION) REGULATION UNLESS OTHERWISE STATED IN THE DRAWING.

# NOTES ON SITE SUPERVISION

THE TCP T5 SITE SUPERVISION PERSONNEL UNDER THE RGE'S STREAM SHALL SUBMIT REGULAR REPORTS OF HER/HIS/THEIR FINDINGS AND RECOMMENDATIONS TO THE RGE. THE RGE SHALL FORMALLY SUBMIT THESE REPØRTS TO THE BD AND PROVIDE A COPY TO THE GEO AT MONTHLY INTERVALS OR MORE PREQUENTLY AS NECESSARY TYPICAL CONTENTS OF THE REGULAR REPORTS PREPARED BY THE TCP T5 SITE SUPERVISION PERSONNEL INCLUDE THE FOLLOWING:

(1) PROGRESS OF THE WORKS (2) RESULTS OF MONITORING DURING CONSTRUCTION (3) SITE OBSERVATIONS (4) INSPECTION RECORDS /

(5) REVIEW

#### STANDARD FOR FILLING WORK

- 1. FILL MATERIAL SHALL BE GRADED, CONTAINING NO PARTICLES COARSER THAN 200mm AND THE PERCENTAGE BY MASS PASSING 75mm BS TEST SIEVE SHALL BE 75% TO
- 2. THE IN SITU FIELD DRY DENSITIES OF COMPACTED MATERIALS FORMING THE EARTH FILL SLOPE SHALL BE NOT LESS THAN 95% OF THE MAXIMUM DRY DENSITY DESCRIBED IN ITEM (2) BELOW.
- 3. THE MAXIMUM DRY DENSITY AND OPTIMUM MOISTURE CONTENTS SHALL BE DETERMINED IN ACCORDANCE WITH THE STANDARD GIVEN IN GEO SPEC 3 CLAUSE 10.1 & 10.2. EACH SOIL TYPE SHALL BE TESTED WHEN FIRST USED THEREAFTER AT THE SAME TIME AS EVERY SET OF FIELD DENSITY TESTS ARE OBTAINED. RECORDS SHALL BE KEPT, IDENTIFYING ON DRAWINGS THE SOIL TYPE, PLAN LOCATION AND ELEVATION REFERENCE TO PRINCIPAL DATUM OF EACH TEST TOGETHER WITH THE MAXIMUM DRY DENSITY AND OPTIMUM MOISTURE CONTENTS. GRAPHS OF DRY DENSITY VS MOISTURE CONTENTS, LABORATORY TEST RECORD SHEETS AND A COMPLETE SOIL DESCRIPTION ARE TO BE KEPT IN A COMPANION FOLDER.
- 4. THE IN SITU FIELD DENSITY AND MOISTURE CONTENTS SHALL BE DETERMINED IN ACCORDANCE WITH THE STANDARD GIVEN IN GEO SPEC 3 CLAUSE 11.1 & PNAP 55 TO DETERMINE THE RELATIVE COMPACTION ACHIEVED. THE NUMBER OF DETERMINATIONS FOR EACH BATCH OF FILL MATERIAL SHALL BE AS STATED IN TABLE 1 BELOW. RECORDS SHALL BE KEPT, IDENTIFYING ON DRAWINGS THE SOIL TYPE, PLAN LOCATION AND ELEVATION REFERENCE TO PRINCIPAL DATUM OF EACH TEST TOGETHER WITH DRY DENSITY OF SOIL TESTED, MOISTURE CONTENTS AND RELATIVE COMPACTION ACHIEVED (%). THE FIELD SHEETS, CALCULATION SHEETS AND A COMPLETE SOIL DESCRIPTION ARE TO BE KEPT IN A COMPANION FOLDER.
- 5. ALL TESTS SHALL BE CARRIED OUT BY OR UNDER THE SUPERVISION OF THE GEOTECHNICAL ENGINEER, OR BY AN INDEPENDENT TESTING AGENCY.

#### NOTES ON PROTECTION OF EARTHWORKS AGAINST HEAVY RAINFALL

- 1. SURFACE WATER FLOWING INTO AND OUT OF THE SITE SHALL BE INTERCEPTED AND CONDUCTED FROM THE SITE TO AN INDICATED SAFE DISCHARGE POINT. AT EACH INTERSECTION AND ABRUPT CHANGE IN DIRECTION OF SURFACE DRAINAGE, CHANNELS AND ACCESSIBLE CATCH PIT SHALL BE PROVIDED. ALL DRAINAGE WORKS SHALL BE KEPT CLEAR
- 2. WHERE PARTIALLY COMPLETED DRAINAGE WORKS DISCHARGE WORKS DISCHARGE WITHIN THE SITE, A TEMPORARY CONDUIT SHALL BE PROVIDED TO THE DISCHARGE POINT.
- 3. DURING EXCAVATION, A METHOD OF WORKING SHALL BE ADOPTED IN WHICH THE MINIMUM AMOUNT OF BARE SOIL IS EXPOSED AT ANY TIME. EXCAVATION TO FORM THE FINAL FACE SHALL BE FOLLOWED UP IMMEDIATELY WITH SURFACE PROTECTION AND DRAINAGE WORKS AND THE FACE PANEL SIZE SHALL BE SMALL ENOUGH TO PERMIT THIS.
- 4. WHERE TEMPORARY BARE EARTH SLOPE FACES ARE UNAVOIDABLE, THEY SHALL BE PROTECTED WITH HEAVY DUTY SHEETING ADEQUATELY SECURED AT THE EDGES, SEALED AT THE CREST, AND LAPPED AT JOINTS. WHERE SLOPE FACES ARE TO BE TEMPORARILY EXPOSED FOR MORE THAN TWO WEEKS, TEMPORARY DRAINS SHALL BE INSTALLED IN ADDITION TO SURFACING.
- TRENCHES ON/OR ADJACENT TO SLOPES SHALL BE EXCAVATED WITH EXTREME CARE IN SHORT SECTIONS AT A TIME. PRECAUTIONS SHALL ALWAYS BE TAKEN TO PREVENT WATER ENTERING AND CONNECTING IN THE TRENCHES.

- NOTES ON SHEET PILING FOR REFERENCE ONLY STEEL SHEET PILES TO COMPLY WITH BS EN 1993-5 2007 GRADE S355. UPON COMPLETION OF INSTALLING SHEET PILE WALLS, A RECORD PLAN FOR SHEET PILES SHALL BE SUBMITTED TO THE BUILDING AUTHORITY VIA THE R.S.E. FOR CONSENT
- IN CASE ROCK OR OBSTRUCTION DUE TO BOULDER OR CORESTONE IS ENCOUNTERED, PREBORING SHOULD BE CARRIED OUT.
- 4. TOLERANCE THE MAXIMUM PERMISSIBLE DEVIATION FROM THE VERTICAL AT ANY LEVEL OF A FINISHED PILE IS 1 IN 75.
- 5. THE SHEET PILE WALLS SHALL BE INSTALLED BY PRESS-IN, NO VIBRO DRAWING IS ALLOWED DURING INSTALLATION

# NOTES ON EXISTING SERVICES, UTILITIES AND STRUCTURES

- 1. BEFORE CONSTRUCTION COMMENCES, THE CONTRACTOR SHALL CONSULT THE VARIOUS SERVICES AND UTILITY AUTHORITIES FOR THE EXTENT OF WORKS TO BE CARRIED OUT.
- 2. THE CONTRACTOR SHALL EXERCISE DUE CARE DURING THE WORKS ON SITE TO AVOID CAUSING DAMAGE TO ADJACENT STRUCTURES PAVETMENT, UTILITIES/SERVICES, PRIVATE AND GOVERNMENT PROPERTIES.
- 3. SHOULD ANY DAMAGE OCCUR TO THE ADJACENT STRUCTURES, PAVEMENT, UTILITIES/SERVICES.
- PRIVATE AND GOVERNMENT PROPERTIES DUE TO THE CONTRACTOR'S WORKS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY COST INCURRED FROM THE DAMAGE. THE CONTRACTOR SHALL REPAIR. REINSTATE AND MAKE GOOD ANY DAMAGE DUE TO THE CONTRACTOR'S WORKS TO THEIR ORIGINAL CONDITIONS OR TO THE SATISFACTION OF THE CM, UNLESS OTHERWISE SPECIFIED.

# PRECAUTIONARY MEASURES TO PREVENT THE

# OCCURRENCE OF OVER BREAK DURING PREBORING

- 1. A PROCEDURE SHALL BE CARRIED OUT TO MONITOR THE CONDITION OF OVER BREAK. IF THE DRILL BIT IS FOUND NOT TO PROPAGATE AFTER A CONSIDERABLE AMOUNT OF DRILLING, THE OPERATOR OF THE DRILLING RIG SHALL STOP THE DRILLING PROCESS AND INFORM THE ENGINEER IMMEDIATELY. THE RGE/RSE SHALL REVIEW THE GEOLOGY OF THE SPECIFIC LOCATION. PROPOSAL TO LIMIT ANY OVER BREAK OF SOIL SHALL BE SUBMITTED TO AND AGREED BY THE RSE/RGE PRIOR TO ANY FURTHER DRILLING WORKS MAY COMMENCE.
- 2. SHOULD ANY UNDUE OVER BREAK OF SOIL OBSERVED DURING THE DRILLING OPERATIONS, THE DRILLING AT THAT LOCATION SHOULD BE STOPPED AND THE RSE SHALL BE INFORM IMMEDIATELY. THE MONITORING DATA AND METHOD OF PREBORING SHALL BE REVIEWED. PROPOSAL TO LIMIT ANY FURTHER OVER BREAK OF SOIL SHALL BE SUBMITTED AND AGREED WITH RSE PRIOR TO ANY FURTHER DRILLING WORKS MAY COMMENCE.

# PRECAUTIONARY MEASURES FOR PREBORING METHOD

- . (a) THE AMOUNT OF AIR SUPPLY TO LIMIT THE PRESSURE OF DRILLINGS SHOULD BE
- (b) THE ADVANCEMENT RATE OF DRILL BIT SHOULD BE MONITORED DURING THE BORING.
- THE OVERBREAK SHOULD NOT BE ALLOWED. 3. THE DRILL BIT SHOULD BE ADVANCED SIMUTANOUSLY WITH THE STEEL CASING.

# DEPROPPING SEQUENCE OF STRUTS

ALL STRUT SHALL NOT BE REMOVED UNTIL CONSTRUCTION UP TO THE GROUND FLOOR OF THE SUPERSTRUCTURE HAS BEEN COMPLETED AND THE REQUIRED 28-DAY CONCRETE STRENGTH HAS BEEN ACHIEVED.

STAGE 1: CAST PILE CAPS, STRAP/ GROUND BEAM (UNDER SEPARATE SUBMISSION) STAGE 2: CAST BASEMENT WALL, COLUMN, WALL, BEAM & SLAB OF B1/F & G/F

(UNDER SEPARATE SUBMISSION) STAGE 3: REMOVE ALL STRUTS WHEN G/F SLAB AND BASEMENT WALL ACHIEVE

28 DAYS OF STREMGTH

#### NOTES ON PRE-BORING FOR INSTALLATION OF SHEET PILES

- 1. THE PRE-BORED HOLES/SHALL BE SUNK ALONG THE ALIGNMENT OF THE SHEET PILE WALL USING SYMMETR/IX DRILLING METHOD. THE PRE-BORED HOLES SHALL BE SUPPORTED BY TEMPORARY STEEL CASING ALONG THE FULL DEPTH OF THE EXCAVATION. 2. THE PRE-BORED HOLES SHALL BE DRILLED IN ACCORDANCE WITH THE FOLLOWING
- REQUIREMENTS: a) DEVIATION FROM THE CORRECT LINE FOR THE LOCATION NOT
- GREATER THAN 20mm. b) DEVIATION F/ROM VERTICALITY OF INDIVIDUAL PRE-BORED HOLES IN ANY DIRECTION SHALL BE LESS THAN 1:100.
- c) DRILL 250mm MINIMUM DIAMETER HOLES FROM EXISTING GROUND LEVEL TO THE REQUIRED LEVEL BY SYMMETRIX DRILLING METHOD.
- 3. AFTER DRILL/ING THROUGH TO THE REQUIRED DEPTH OF OBSTRUCTIONS THE INTERIOR OF EACH CASING SHALL BE FILLED WITH APPROVED GRANULAR BACKFILL MATERIAL/SHALL BE TOPPED UP IMMEDIATELY.
- 4. UPON COMPLETION SHEET PILE WALL SHALL BE INSTALLED TO THE REQUIRED TOE LEVEL BY THE METHOD APPROVED BY THE RSE. THROUGH A GUIDE FRAME AT GROUND LEVEL TO ENSURE PROPER PITCHING, VERTICALITY AND ALIGNMENT OF SHEET PILE WALL
- 5. NO WITHSTANDING THE ABOVE-MENTIONED MINIMUM PRE-BORING REQUIREMENTS, IT IS/THE CONTRACOTR'S RESPONSIBILITY TO PROVIDE ANY ADDITIONAL PRÉ-BORING OR ALTERNATIVE MEASURES TO ENSURE THAT ALL SHEET PILE WALLS ARE TO BE PRESSED IN FREE OF OBSTRUCTIONS TO ACHIEVE THE REQUIRED TOE
- 6. THE CONTRACTOR SHALL SUBMIT A DETAILED METHOD STATEMENT TOGETHER WITH THE PLANT AND EQUIPMENT FOR PRE-BORING to AP, RSE & RGE FOR APPROVAL BEFORE COMMENCEMENT OF WORKS. THE PROPOSED METHOD AND SEQUENCE OF PRE-BORING SHALL BE ARRANGED SO AS TO MINIMIZE THE CONSTRUCTION NOISE
- SHALL ANY UNDUE SETTLEMENT OCCUR DUE TO PRE-BORING, THE CONTRACTOR SHALL SUBMIT A REMEDIAL PROPOSAL FOR THE APPROVAL OF THE RSE TO PREVENT FURTHER UNDUE SETTLEMENT PRIOR TO THE RE-COMMENCEMENT OF THE PRE-BORING WORKS.

8. THE CONTRACTOR SHALL KEEP RECORD OF EACH PRE-BORED HOLES FOR

# **SOIL PARAMETER**

ENGINEER INSPECTION.

	SOIL PARAMETER	
	Ø' (DEGREE)	C' (kpa)
FILL	30	0
MD	33	1
ALL.	32	2
CDG	34	5

#### SCHEDULE OF VERTICAL TIE

	SCHEDULE (	OF VERTICAL TIE	
ITEM	MEMBER MARK	GRADE	MEMBER SIZE
VERTICAL_TIE	P4	S355	UBP356x368x174

#### SCHEDULE OF HORIZONTAL TIE

	SCHEDULE OF	HORIZONTAL TIE	
ITEM	MEMBER MARK	GRADE	MEMBER SIZE
HORIZONTAL_TIE	T1	S355	UC203x203x46
HORIZONTAL_TIE	T2	S355	UC203x203x46
HORIZONTAL_TIE	T3	S355	UC203x203x46
HORIZONTAL_TIE	T4	S355	UC203x203x46
HORIZONTAL_TIE	T5	S355	UC203x203x46
HORIZONTAL_TIE	T6	S355	UC203x203x46
HORIZONTAL_TIE	T7	S355	UC203x203x46

# SCHEDULE OF MAIN STRUT

		SCHEDUL	E OF MAIN STRI	JT	
PILE TYPE	LAYER	STRUT MEMBER SIZE	STRUT LEVEL	HORIZONTAL LOAD (kN/m)	DESIGN LOAD FOR STRUT (kN)
Α	1	UC305x305x97	+3.254	86	569
Α	2	UC305x305x97	+1.754	130	860
Α	3	UC356x368x177	+0.284	251	1661
Α	4	UC356x368x177	-1.216	452	2990
Α	5	UC356x368x202	-2.713	640	4234
Α	6	UC356x406x235	-4.210	824	5451
Α	7	UC356x406x287	-5.703	805	5326
Α	8	UC356x406x287	-7.203	961	6358
В	1	UC305x305x97	+3.254	156	1032
В	3	UC356x368x177	+0.284	410	2713
В	5	UC356x368x177	-2.716	411	2719
В	6	UC356x368x202	-4.213	600	3969
В	7	UC356x368x202	-5.713	623	4122
В	8	UC356x406x235	-7.210	528	3493
С	1	UC305x305x97	+3.254	130	860
С	3	UC356x368x177	+0.284	420	2779
С	5	UC356x368x202	-2.713	673	4452
С	7	UC356x406x287	-5.703	1032	6827

# - DRILL PIPE AIR AND **CUTTINGS FLOW** TEMPORARY CASING WITH INDERENDENT ROTARY ACTION RETRACTABLE DRILL BIT - RING BIT

#### SCHEMATIC DETAILS OF RING BIT DRILLING SYSTEM FOR OVERCOMING UNDERGROUND OBSTRUCTIONS

N.T.S.

# SCHEDULE OF WALING

		SCH	EDULE OF WAL	ING	
PILE	LAVED	WALING MEMBER CITE	COMPRESSION (kNm)	SHEAR (kNm)	MOMENT (kNm)
YPE	LAYER	WALING MEMBER SIZE	=1.4 x Fh x (1.414 x 3.15)	=1.4 x Fh x (0.6 x 3.15)	=1.4 x Fh x (3.15^2/9)
Α	1	UB533x210x92	464	199	87
Α	2	UB533x210x92	803	344	151
Α	2	UB533x210x92	1186	509	223
Α	3	UB610x305x179	2538	1088	476
Α	4	UB610x305x179	2791	1196	524
Α	4	UB610x305x179	3020	1294	567
Α	5	UB610x305x238	2538	1088	476
Α	6	UB610x305x238	3705	1588	695
Α	7	UB610x305x238	3874	1649	722
Α	8	UB914x305x289	3260	1398	612
AA & D	1	UB533x210x92	464	199	87
AA & D	2	UB533x210x92	1186	509	223
AA & D	3	UB610x305x179	2538	1088	476
AA & D	4	UB610x305x238	3020	1294	567
AA & D	5	UB610x305x238	2538	1088	476
AA & D	6	UB610x305x238	3705	1588	695
AA & D	7	UB610x305x238	3874	1649	722
В	1	UB533x210x92	464	199	87
В	3	UB610x305x179	2538	1088	476
В	5	UB610x305x179	2538	1088	476
В	6	UB610x305x238	3705	1588	695
В	7	UB610x305x238	3874	1649	722
В	8	UB610x305x238	3260	1398	612
В	8	UB914x305x289	3260	1398	612
С	1	UB533x210x92	464	199	87
С	3	UB610x305x179	2538	1088	476
С	5	UB610x305x238	2538	1088	476
С	7	UB914x305x289	3874	1649	722

# <u>SCHEDULE OF SECONDARYARY STRUT AND CORNER STRUT</u>

	ULE OF	SECONDARY STRUT AND (	CORNER STRUT
PILE TYPE	LAYER	STRUT MEMBER SIZE	STRUT LEVEL
Α	1	UC356x368x202	+3.206
Α	2	UC356x368x202	+1.787
Α	3	UC356x368x202	+0.287
Α	4	UC356x368x202	-1.213
Α	5	UC356x406x235	-2.710
Α	6	UC356x406x235	-4.210
Α	7	UC356x406x235	-5.710
Α	8	UC356x406x235	-7.210
AA & D	1	UC356x368x202	+3.206
AA & D	2	UC356x368x202	+1.787
AA & D	3	UC356x368x202	+0.287
AA & D	4	UC356x368x202	-1.213
AA & D	5	UC356x406x235	-2.710
AA & D	6	UC356x406x235	-4.210
AA & D	7	UC356x406x235	-5.710
В	1	UC356x368x202	+3.206
В	3	UC356x368x202	+0.287
В	5	UC356x368x202	-2.713
В	6	UC356x368x202	-4.213
В	7	UC356x406x235	-5.710
В	8	UC356x406x235	-7.210
С	1	UC356x368x202	+3.202
С	3	UC356x368x177	+0.284
С	5	UC356x368x177	-2.716
С	7	UC356x368x202	-5.713

#### SECTION PROPERTIES OF WALING

			SECTION	<b>PROPERT</b>	IES OF WAL	.ING			
ITEM	GRADE	SECTION AREA (cm?	MOMENT OF INERTIA (cm4)	WEIGHT (kg/m)	SECTION MODULUS (cm?	DEPTH D (mm)	WIDTH B (mm)	WEB THICKNESS t (mm)	FLANGE THICKNESS T (mm)
UB533x210x92	S355	118	55333	92	2076	533.1	209.3	10.1	15.6
UB610x305x179	S355	228	151434	178	4906	620.2	307.1	14.1	23.6
UB610x305x238	S355	304	207747	238	6564	635.8	311.4	18.4	31.4
UB914x305x289	S355	369	504850	289	10897	926.6	307.7	19.5	32.0

#### SECTION PROPERTIES OF STRUTS

			SECTION	PROPERT	IES OF STR	UTS			
ITEM	GRADE	SECTION AREA (cm?	MOMENT OF INERTIA (cm4)	WEIGHT (kg/m)	SECTION MODULUS (cm?	DEPTH D (mm)	WIDTH B (mm)	WEB THICKNESS t (mm)	FLANGE THICKNESS T (mm)
UC305x305x97	S355	123	22184	96	1442	307.9	305.3	9.9	15.4
UC356x368x177	S355	226	57040	177	3099	368.2	372.6	14.4	23.8
UC356x368x202	S355	258	66255	202	3538	374.6	374.7	16.5	27.0
UC356x406x235	S355	300	79103	235	4153	381.0	394.8	18.4	30.2
UC356x406x287	S355	366	99817	287	5073	393.6	399.0	22.6	36.5

SECTION PROPERTIES OF SHORT STRUT / SPACER

(kg/m)

WEIGHT MODULUS

29.05 153

(cm?

(mm)

152.4 88.9

DEPTH D | WIDTH B | THICKNESS T | THICKNESS T

7.1

(mm)

(mm)

SECTION PROPERTIES OF HORIZONITAL TIE

C 152x89x24

SECTION PROPERTIES OF SHORT STRUTS/ SPACER

GRADE

S355

SECTION AREA

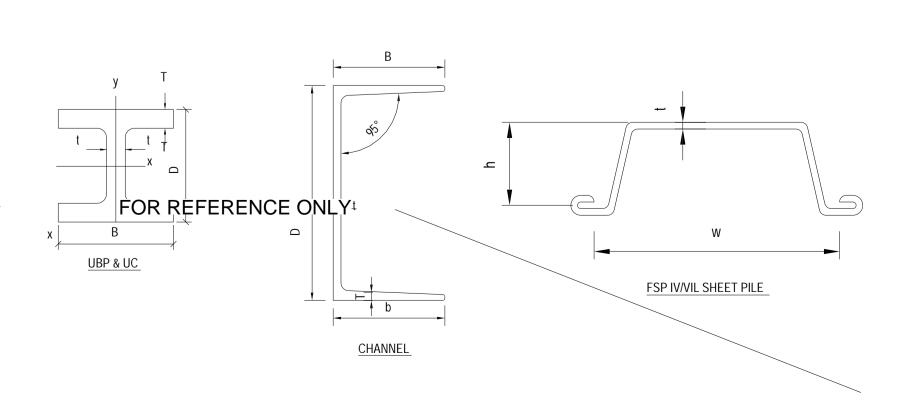
30.4

<u> </u>	LCHON FROFERIES	OI HORIZ	LONTAL TIL							
			SI	ECTION PROPER	TIES OF H	ORIZONTAL	TIE			
	ITEM	GRADE	SECTION AREA (cm?	MOMENT OF INERTIA (cm4)	WEIGHT (kg/m)	SECTION MODULUS (cm?	DEPTH D (mm)	WIDTH B (mm)	WEB THICKNESS t (mm)	FLANGE THICKNESS T (mm)
	UC203x203x46	S355	58.8	4565	46.18	449	203.2	203.6	7.2	11.0

1168

# SECTION PROPERTIES OF VERTICAL TIE

		SE	ECTION PROPER	TIES OF VE	ERTICAL TIE				
ITEM	GRADE	SECTION AREA (cm?	MOMENT OF INERTIA (cm4)	WEIGHT (kg/m)	SECTION MODULUS (cm?	DEPTH D (mm)	WIDTH B (mm)	WEB THICKNESS t (mm)	FLANGE THICKNESS T (mm)
UBP356x368x174	S355	221.5	5100	172.32	2820	361.4	378.5	20.3	20.4



BD REF BIM REF

> AMENDMENT CIC SAMPLE PROJECT

# **EXCAVATION & LATERAL SUPPORT** GENERAL NOTES

SCALE AS SHOWN@A1 DRAWING NO.

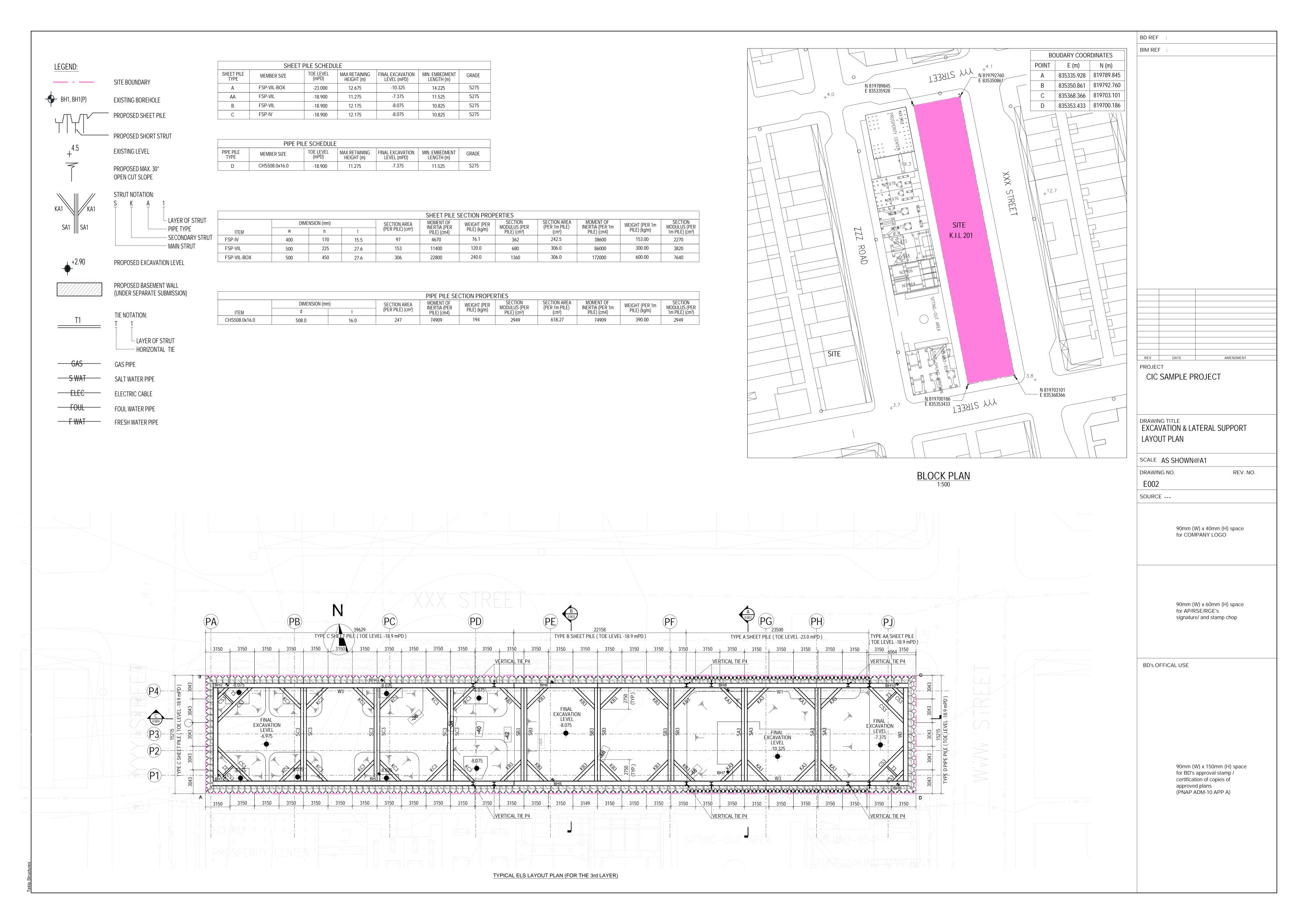
> 90mm (W) x 40mm (H) space for COMPANY LOGO

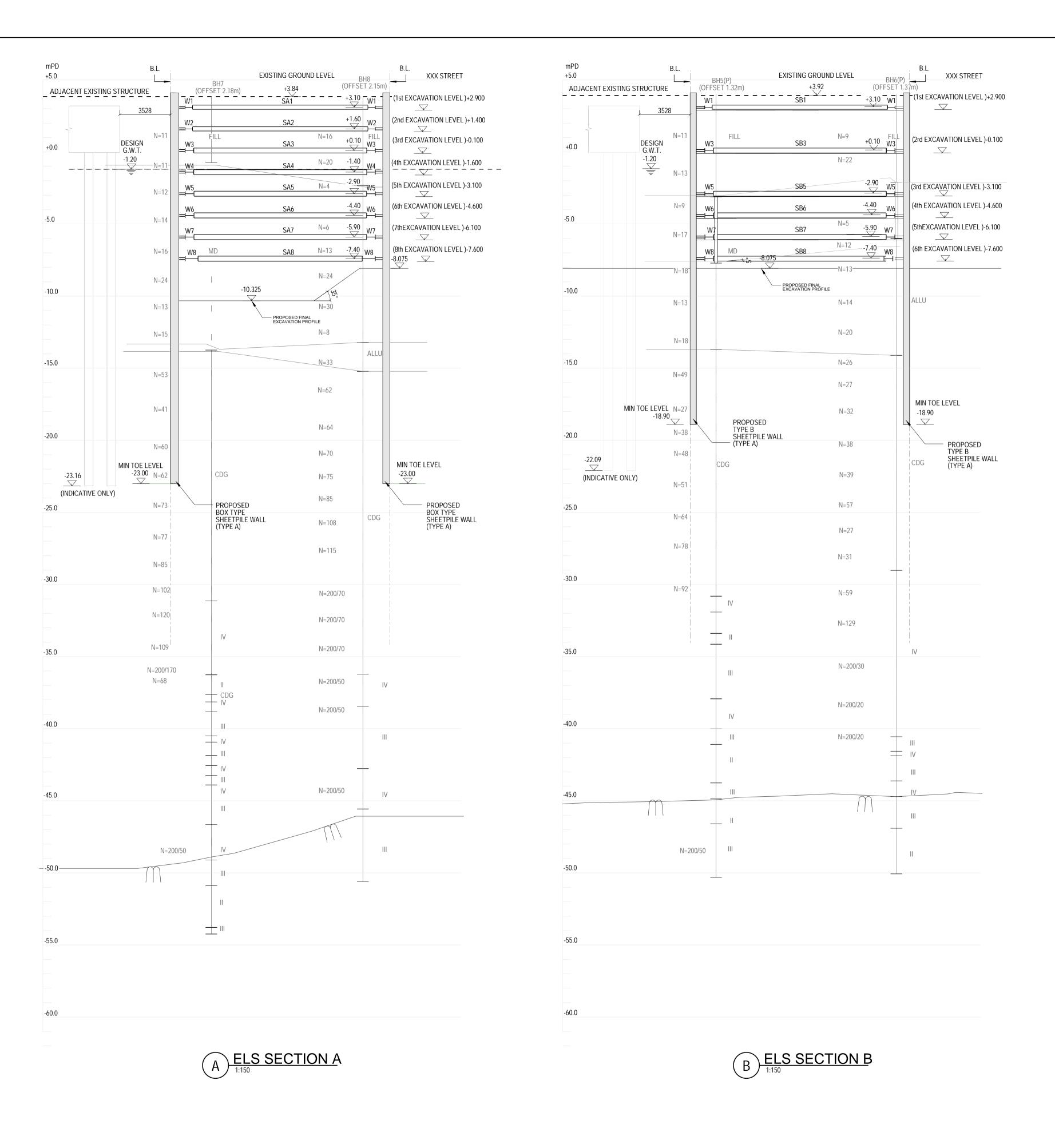
90mm (W) x 60mm (H) space for AP/RSE/RGE's signature/ and stamp chop

**BD's OFFICAL USE** 

SOURCE ---

90mm (W) x 150mm (H) space for BD's approval stamp / certification of copies of approved plans (PNAP ADM-10 APP A)





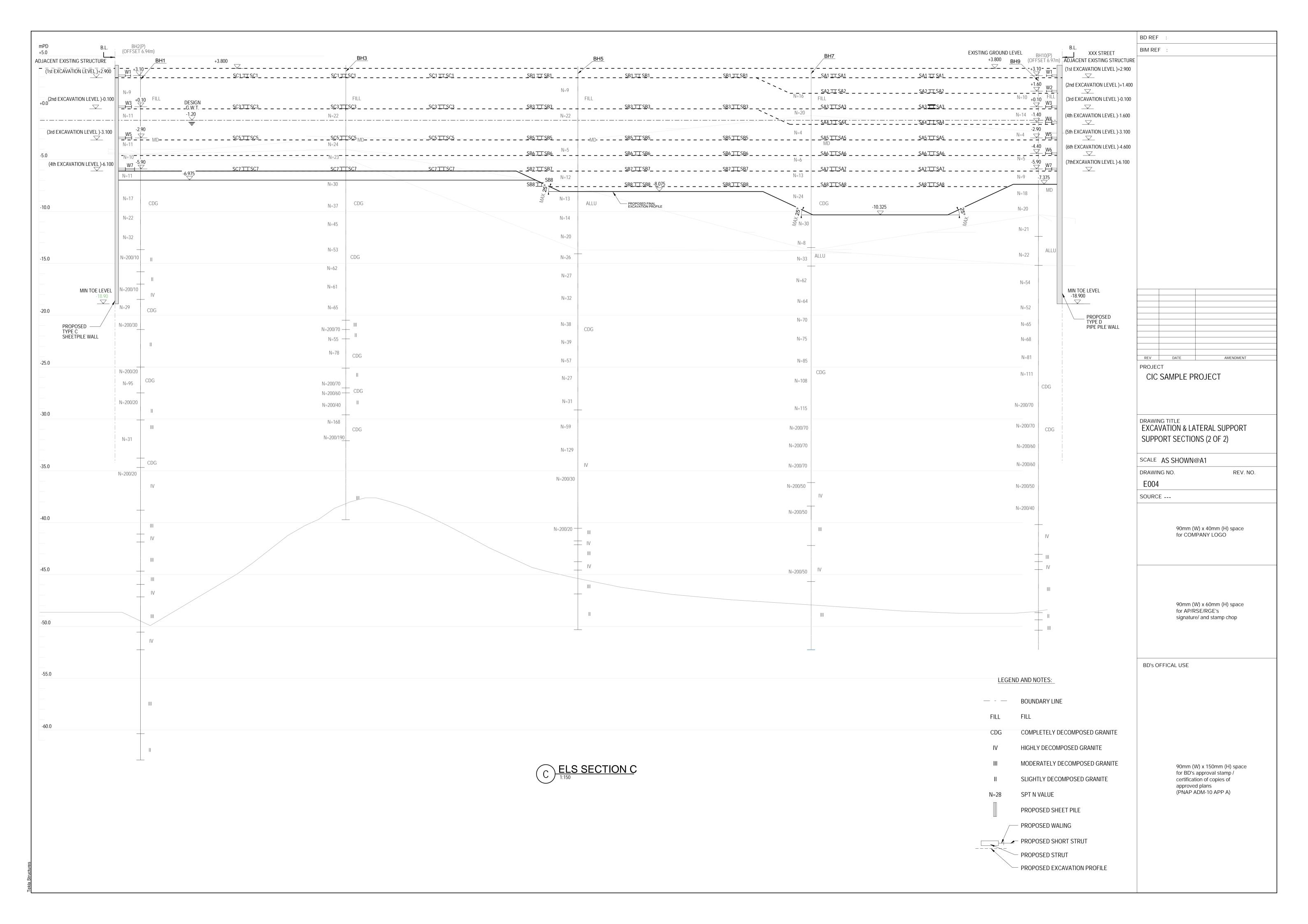
LEGEND	AND NOTES:
	BOUNDARY LINE
FILL	FILL
CDG	COMPLETELY DECOMPOSED GRANITE
IV	HIGHLY DECOMPOSED GRANITE
III	MODERATELY DECOMPOSED GRANITE
II	SLIGHTLY DECOMPOSED GRANITE
N=28	SPT N VALUE
	PROPOSED SHEET PILE
	PROPOSED WALING
	PROPOSED SHORT STRUT
	PROPOSED STRUT
	PROPOSED EXCAVATION PROFILE

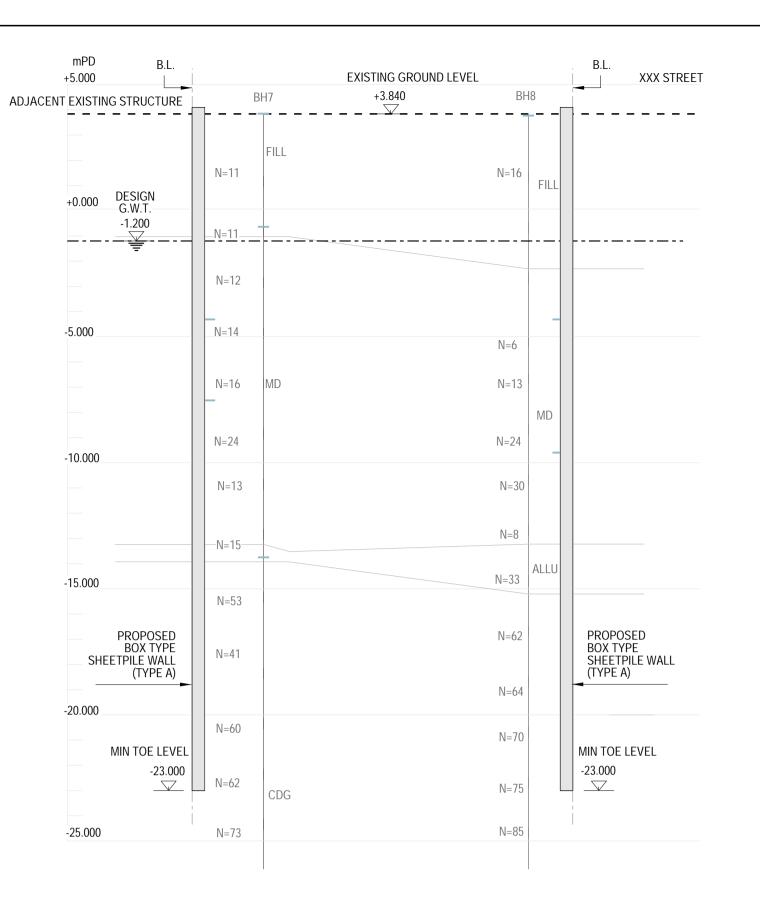
REV	DATE	
PROJEC	СТ	AMENDMENT
	SAMPLE PI	
CIC DRAWIN EXCA	SAMPLE PI	ROJECT
DRAWIN EXCA SUPP	SAMPLE PI	TERAL SUPPORT
DRAWIN EXCA SUPP	SAMPLE PI	TERAL SUPPORT
DRAWIN EXCA' SUPP	SAMPLE PI	ROJECT  TERAL SUPPORT  NS (1 OF 2)  @A1
DRAWIN EXCA SUPP SCALE DRAWIN E003	SAMPLE PI	ROJECT  TERAL SUPPORT  NS (1 OF 2)  @A1
DRAWIN EXCA SUPP SCALE DRAWIN E003	SAMPLE PI	ROJECT  TERAL SUPPORT  NS (1 OF 2)  @A1  REV. NO.

BD REF

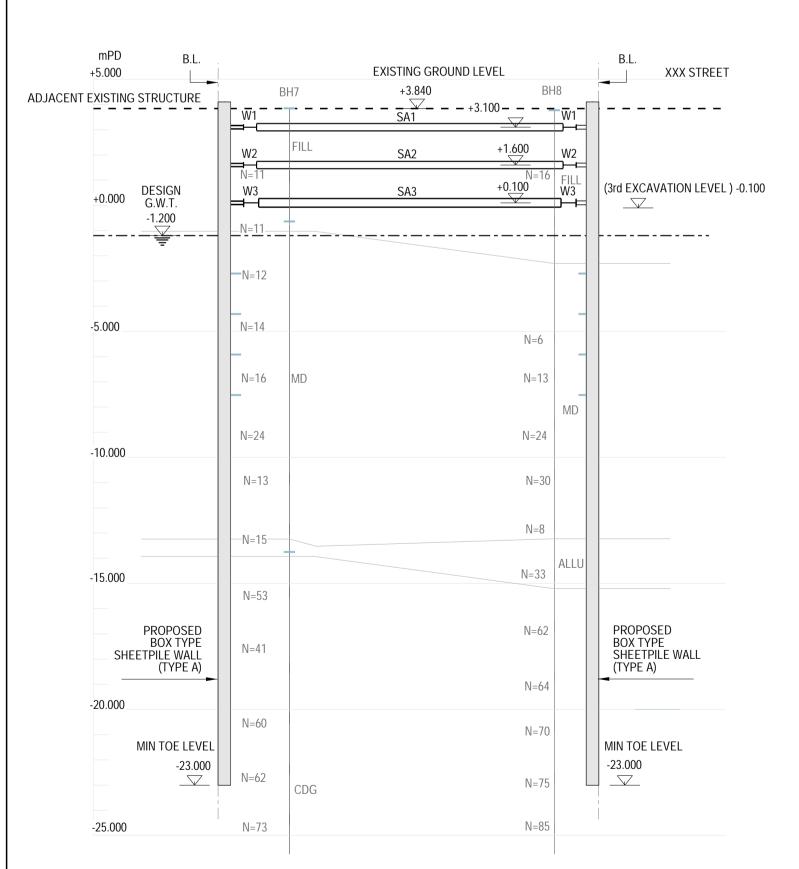
BIM REF

90mm (W) x 150mm (H) space for BD's approval stamp / certification of copies of approved plans (PNAP ADM-10 APP A)





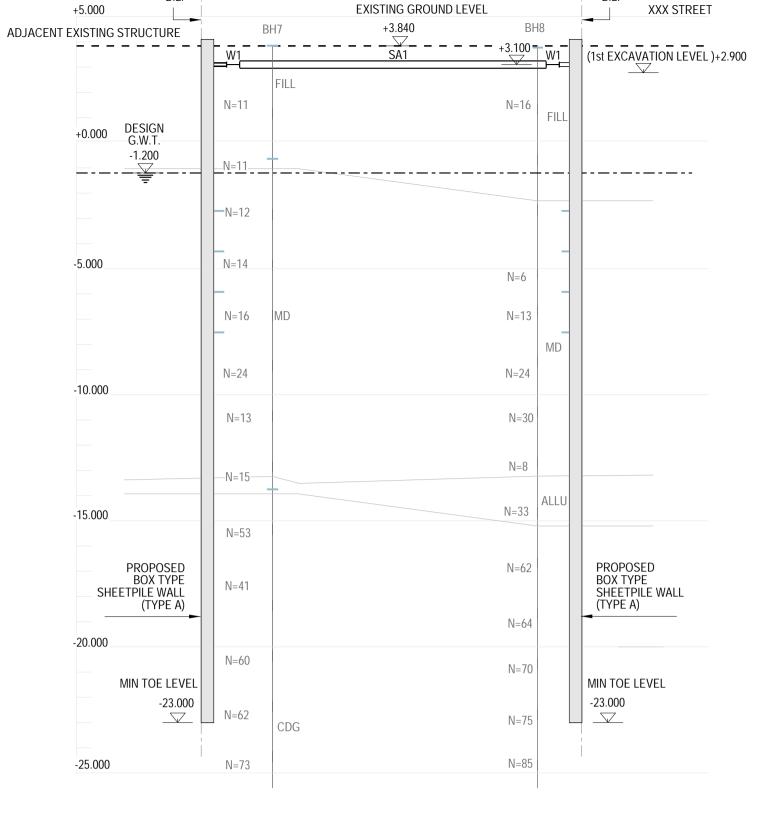
INSTALL MONITORING CHECKPOINTS AS SHOWN ON DRAWING NO. E008 AND TAKE INITIAL READING
 CARRY OUT INSTALLATION OF SHEET PILES AS SHOWN ON PLAN TO REQUIRED LEVEL.
 CARRY OUT PUMPING TEST AS SHOWN ON DWG NO.: E009.



DEWATER AND EXCAVATE TO -0.100mPD.
 INSTALLATION OF THE 3rd LAYER WAILINGS, STRUTS & TIES

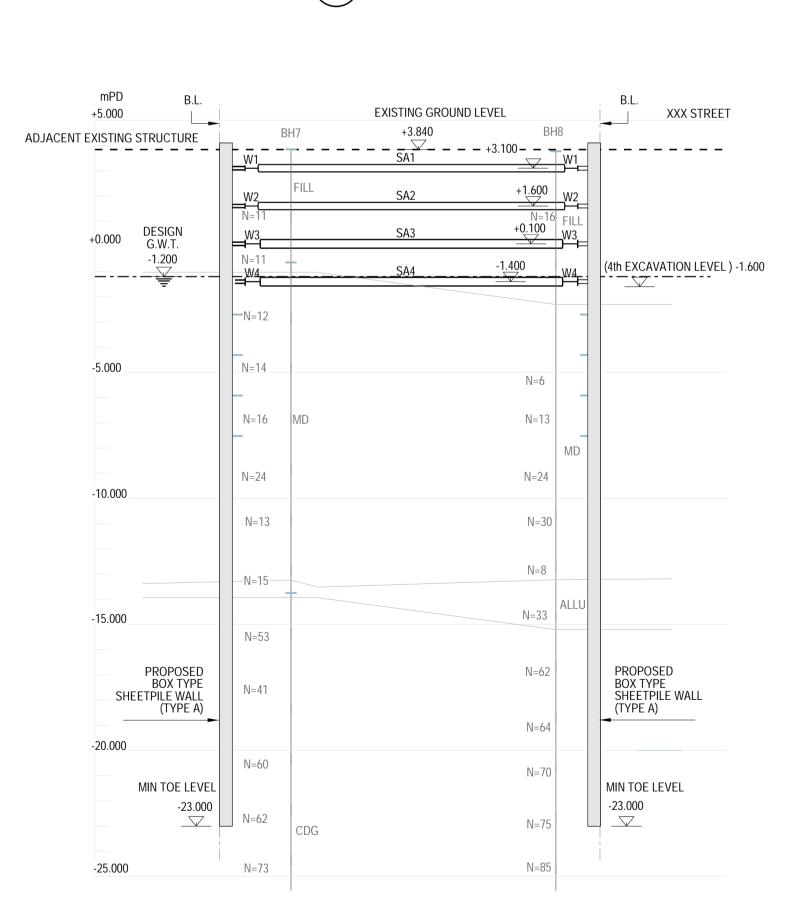


DEWATER AND EXCAVATE TO -1.600mPD.
 INSTALLATION OF THE 4th LAYER WAILINGS, STRUTS & TIES



mPD

DEWATER AND EXCAVATE TO +2.900mPD.
 INSTALLATION OF THE 1st LAYER WAILINGS, STRUTS & TIES



mPD EXISTING GROUND LEVEL +5.000 XXX STREET ADJACENT EXISTING STRUCTURE (2nd EXCAVATION LEVEL )+1.400 +0.000 DESIGN G.W.T. -1.200 -5.000 N=6 N=16 ME -10.000 N=13 N=30 N=8 N=33 -15.000 PROPOSED BOX TYPE PROPOSED BOX TYPE SHEETPILE WALL (TYPE A) SHEETPILE WALL (TYPE A) -20.000 N=70 MIN TOE LEVEL MIN TOE LEVEL -23.000 -23.000 N=85 -25.000 N=73

DEWATER AND EXCAVATE TO +1.400mPD.
 INSTALLATION OF THE 2nd LAYER WAILINGS, STRUTS & TIES



LEGEND AND NOTES: **BOUNDARY LINE** FILL FILL COMPLETELY DECOMPOSED GRANITE HIGHLY DECOMPOSED GRANITE MODERATELY DECOMPOSED GRANITE SLIGHTLY DECOMPOSED GRANITE N = 28SPT N VALUE PROPOSED SHEET PILE PROPOSED WALING ▶ PROPOSED SHORT STRUT PROPOSED STRUT

PROPOSED EXCAVATION PROFILE

FOR SECTION A (1 OF 2) SCALE AS SHOWN@A1 DRAWING NO. REV. NO. E005 SOURCE ---90mm (W) x 40mm (H) space for COMPANY LOGO

AMENDMENT

90mm (W) x 60mm (H) space for AP/RSE/RGE's signature/ and stamp chop

BD's OFFICAL USE

BD REF

BIM REF

PROJECT

DRAWING TITLE

CIC SAMPLE PROJECT

**EXCAVATION & LATERAL SUPPORT** 

CONSTRUCTION SEQUENCE

90mm (W) x 150mm (H) space for BD's approval stamp / certification of copies of approved plans (PNAP ADM-10 APP A)



- 4. CARRY OUT REMAINING PILE CAP CONSTRUCTION (UNDER SEPARATE SUBMISSION) AND BACKFILL TO PILE CAP TOP
  5. CARRY OUT BASEMENT CONSTRUCTION (UNDER SEPARATE SUBMISSION)
  6. ALL STRUT SHALL NOT BE REMOVED UNTIL CONSTRUCTION UP TO G/F.

- DEWATER AND EXCAVATE TO -7.600mPD.
   INSTALLATION OF THE 8th LAYER WAILINGS, STRUTS & TIES
   DEWATER AND EXCAVATE TO FINAL EXCAVATE PROFILE (i.e.: -8.075/-10.325mPD) WITH TEMPORARY CUT SLOPE(30° MAX). (REFER TO ELS LAYOUT PEMAINING PILE CAR TO PLACE TO THE PLACE TO THE
- +0.000 DESIGN G.W.T. -1.200 -1.400 (4th EXCAVATION LEVEL ) -1.600 ----<del>-</del>-----2.900 -4.400 -5.000 -5.900 <sub>N=6</sub> -7.400 N=13 (8th EXCAVATION LEVEL ) -7.600 -10.000 N=13 N=30 — PROPOSED FINAL EXCAVATION PROFILE N=8 —N=15— N = 33-15.000 N=53 PROPOSED BOX TYPE PROPOSED BOX TYPE SHEETPILE WALL (TYPE A) SHEETPILE WALL (TYPE A) -20.000 N=70 MIN TOE LEVEL MIN TOE LEVEL -23.000 -23.000 N=85

+1.600

+0.100

EXISTING GROUND LEVEL

+3.840 

DEWATER AND EXCAVATE TO -3.100mPD.
 INSTALLATION OF THE 5th LAYER WAILINGS, STRUTS & TIES

ADJACENT EXISTING STRUCTURE  $\overline{W}_1 - \overline{\overline{Y}}_1 - \overline{\overline{Y}_$ DESIGN G.W.T. +0.000 -1.200 -2.900 (5th EXCAVATION LEVEL) -3.100 -5.000 N=6 N=13 N=16 MD N=24 -10.000 N=30 N=8 —N=15— N=33 -15.000 N = 53PROPOSED BOX TYPE N=62 **BOX TYPE** SHEETPILE WALL (TYPE A) SHEETPILE WALL (TYPE A) -20.000 MIN TOE LEVEL MIN TOE LEVEL -23.000 -23.000 N=75 N=85 -25.000

EXISTING GROUND LEVEL

XXX STREET

XXX STREET

+5.000

-5.000

-10.000

-15.000

-20.000

-25.000

PROPOSED BOX TYPE

SHEETPILE WALL (TYPE A)

MIN TOE LEVEL

-23.000

+0.000 DESIGN G.W.T.

-1.200

N=16 MI

N=13

mPD

+5.000

+5.000

ADJACENT EXISTING STRUCTURE

-25.000

N=73

DEWATER AND EXCAVATE TO -4.600mPD.
 INSTALLATION OF THE 6th LAYER WAILINGS, STRUTS & TIES

EXISTING GROUND LEVEL

-1.400

-2.900

-4.400

N=24

N=8

N=33

PROPOSED BOX TYPE SHEET PILE WALL

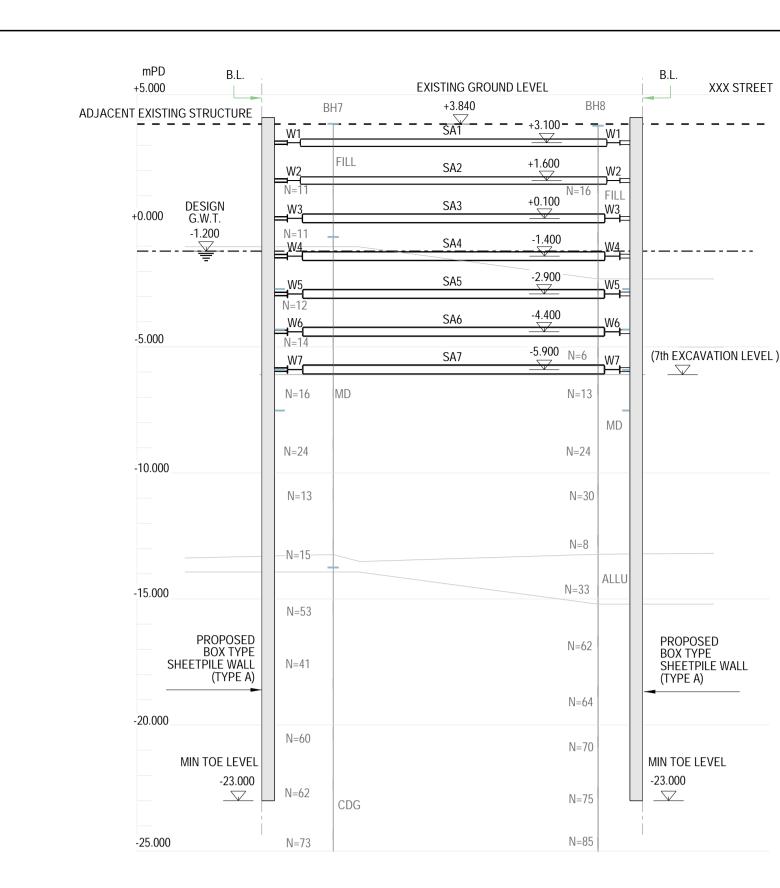
(TYPE A)

MIN TOE LEVEL

-23.000

XXX STREET

(6th EXCAVATION LEVEL) -4.600



DEWATER AND EXCAVATE TO -6.100mPD.
 INSTALLATION OF THE 7th LAYER WAILINGS, STRUTS & TIES

FILL

N = 28

LEGEND AND NOTES:

FILL

**BOUNDARY LINE** 

SPT N VALUE

PROPOSED SHEET PILE

- PROPOSED WALING

- PROPOSED STRUT

▶ PROPOSED SHORT STRUT

PROPOSED EXCAVATION PROFILE

(7th EXCAVATION LEVEL ) -6.100 AMENDMENT PROJECT CIC SAMPLE PROJECT DRAWING TITLE **EXCAVATION & LATERAL SUPPORT** CONSTRUCTION SEQUENCE FOR SECTION A (2 OF 2) SCALE AS SHOWN@A1 DRAWING NO. E006 SOURCE ---

COMPLETELY DECOMPOSED GRANITE

HIGHLY DECOMPOSED GRANITE 90mm (W) x 60mm (H) space for AP/RSE/RGE's signature/ and stamp chop MODERATELY DECOMPOSED GRANITE SLIGHTLY DECOMPOSED GRANITE

BD's OFFICAL USE

for BD's approval stamp /

90mm (W) x 40mm (H) space for COMPANY LOGO

certification of copies of approved plans (PNAP ADM-10 APP A)

90mm (W) x 150mm (H) space

REV. NO.

BIM REF

BD REF

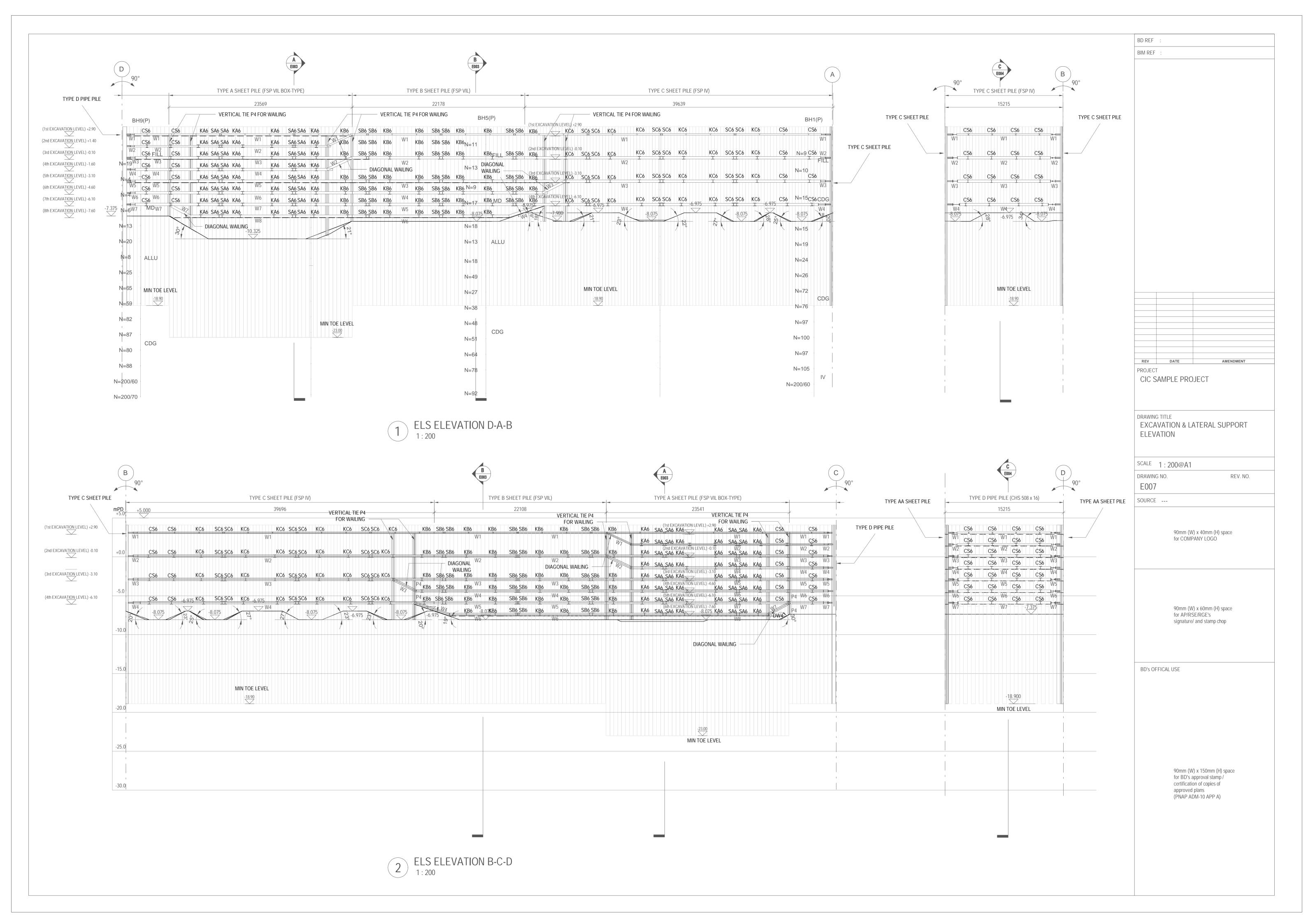
XXX STREET

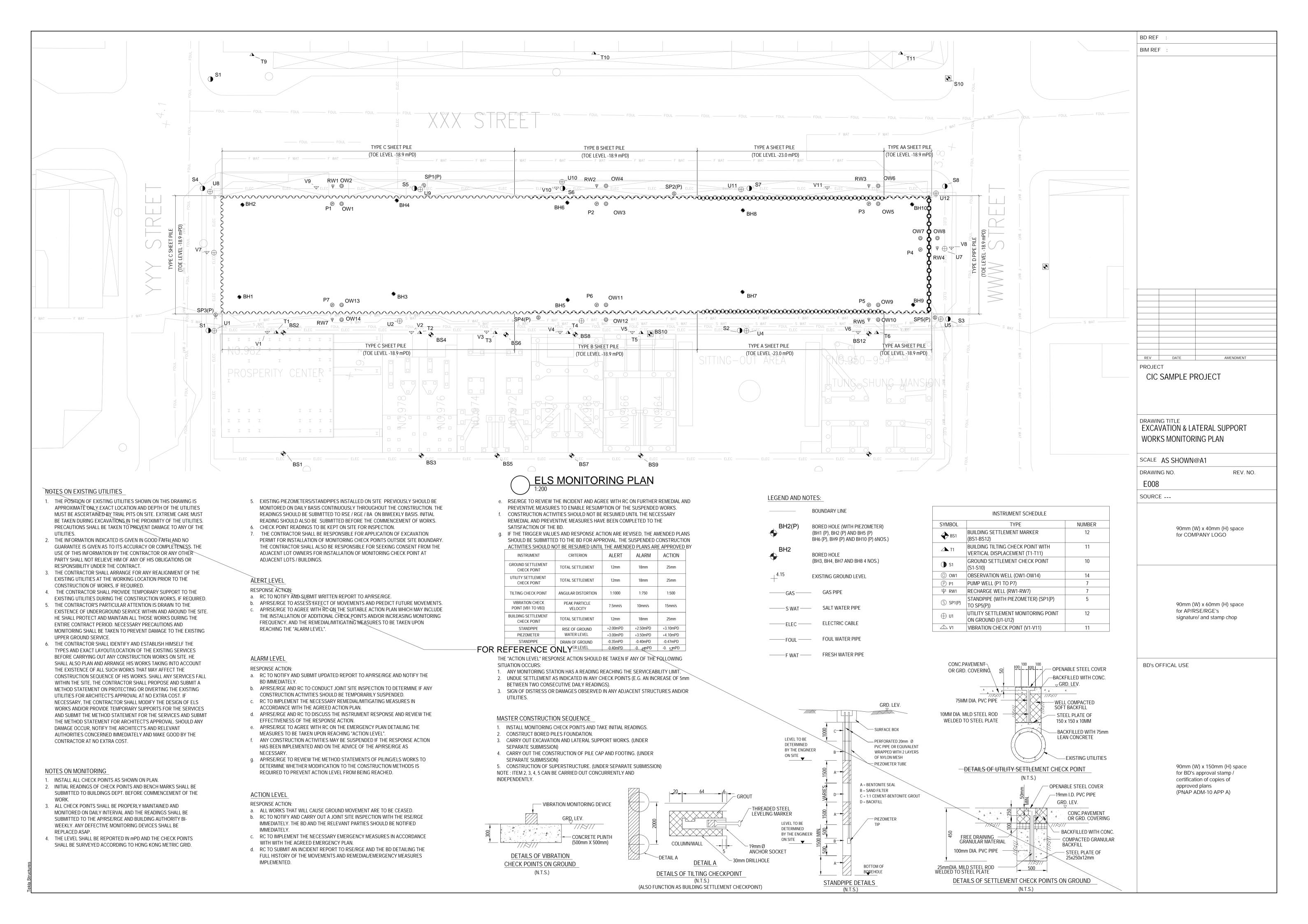
PROPOSED BOX TYPE

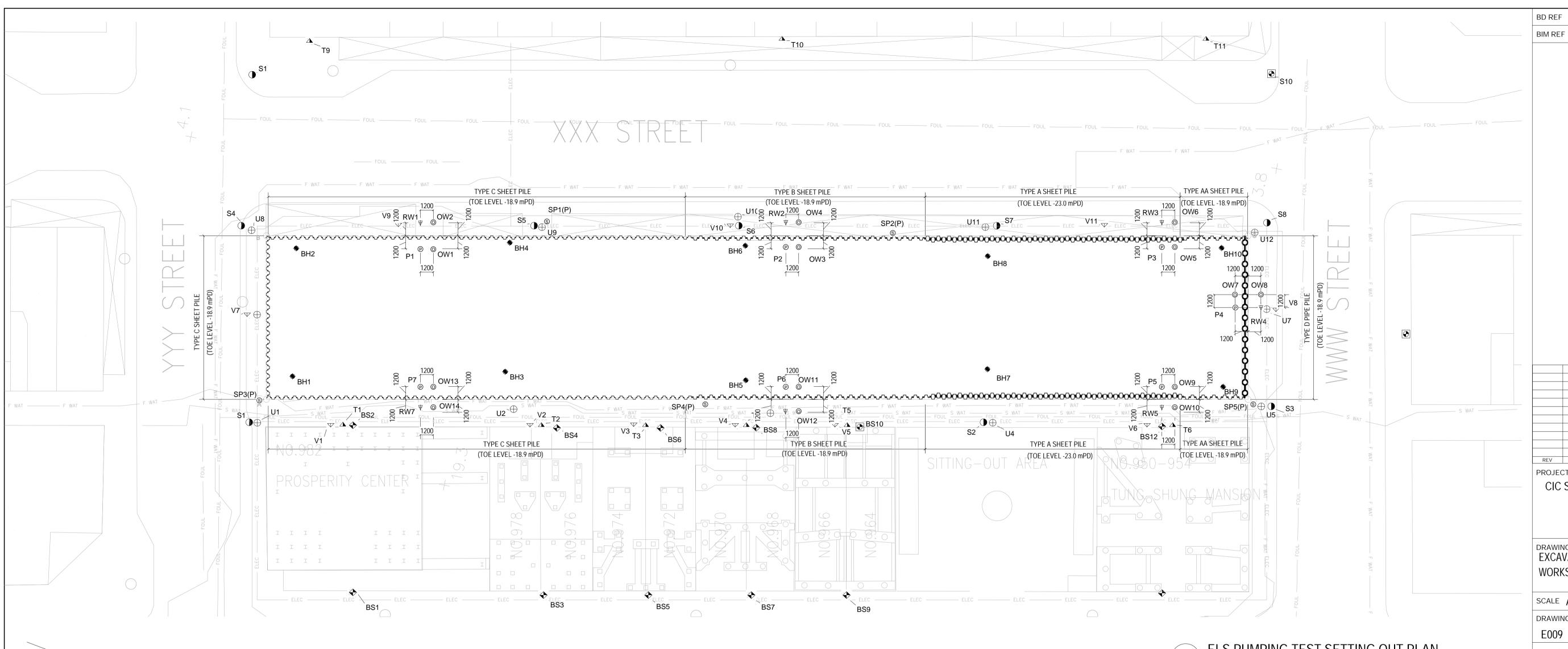
(TYPE A)

-23.000

SHEETPILE WALL







# (I) PUMPING TEST PROCEDURES

- 1. INSTALL THE DEWATERING WELLS (P1-P7), STANDPIPES AND OBSERVATION WELLS (OW1-OW15) AS SHOWN IN DWG. NO.
- 2. THE PROPOSED TOE LEVEL OF THE DEWATERING WELL AND OBSERVATION WELLS ARE AS FOLLOWS:

HOLE	PROPOSED	PROPOSED LEVEL OF WATER LEVEL CONTROL ELECTRODES		
HOLE	TOE LEVEL (mPD)	CUT-ON LEVEL (mPD)	CUT-OUT LEVEL (mPD)	
DEWATERING WELL (P1-P7)	-15.00	-12.50	-13.50	
OBSERVATION WELL (OW1-OW15)	-15.00	N/A	N/A	

- 3. BEFORE INSERTION OF THE SUBMERSIBLE PUMP, THE DEWATERING WELLS SHALL BE CLEANED, FLUSHED AND THE DEPTH OF THE WELL SHALL BE ACCURATELY MEASURED
- 4. THE DEWATERING WELLS INCLUDING DISCHARGE PIPES SHALL THEN BE COMPLETED AND TESTED TO BE FUNCTIONAL 5. THE PROPOSED DEWATERING PUMP TO BE INSTALLED IS 'MASTRA' MODEL R95-S-08 WITH A MINIMUM DISCHARGE
- CAPACITY OF 10CU.M/HR/WELL UNDER A DELIVERY HEAD OF 46M AND BE ABLE TO LOWER WATER LEVEL WITHIN 1M ABOVE THE PUMP. 6. FLOW METER AND GATE VALVE TO CONTROL FLOW SHALL BE INCOPORATED INTO EACH DEWATERING WELL.
- 7. BEFORE COMMENCEMENT OF PUMP TEST, WATER LEVELS IN ALL DEWATERING WELLS, STANDPIPES, OBSERVATION WELLS SHALL BE MEASURED AT 4 HOURS INTERVALS FOR A PERIOD OF 72 HOURS. THE LOWEST MEASURED LEVELS IN THE PUMP WELL AND STANDPIPES SHALL BE USED AS INITIAL READINGS FOR THE PUMPING TEST.
- 8. ALL DEWATERING PUMPS SHALL BE SWITCHED ON SIMULTANEOUSLY. 9. STEADY STATE SHALL BE DEFINED AS SUCH THAT THE RATE OF GROUNDWATER DRAW DOWN BOTH INSIDE AND OUTSIDE THE SITE IS LESS THEN 0.1M OVER AN HOUR.
- 10. THE WATER LEVEL IN THE PUMP WELL SHALL BE MAINTAINED AT THE SPECIFIED LEVEL FOR AT LEAST 72 HOURS. 11. SHOULD THE SPECIFIED STEADY STATE IN GROUNDWATER DRAWDOWN NOT BE REACHED, PUMPING SHALL BE CONTINUED UNTIL SUCH A STATE IS REACHED OR AS DIRECTED BY THE ENGINEER. THE MINIMUM TEST PERIOD IS 7
- 12. DURING THE TEST THE WATER LEVELS IN ALL DEWATERING WELL, OBSERVATION WELLS AS SHOWN ON DWG. NO.
- ELS-13 SHALL BE REACHED AT REGULAR INTERVALS WHICH SHOWN BELOW (II) WATER LEVEL MEASURMENT.
- 13. ALL MONITORING DATA SHALL BE PRODUCED IN BOTH TABULAR AND GRAPHICAL FORM DURING THE COURSE OF THE PUMPING TEST AND SUBMIT TO ENGINEER FOR BUILDINGS DEPARTMENT'S APPROVAL.
- 14. WATER LEVELS SHALL BE MONITORIED AFTER CESSATION OF PUMPING UNTIL RECOVERY TO INITIAL LEVELS IS

# (II) WATER LEVEL MEASUREMENT

DURING THE PUMPING AND RECOVERY TESTS, THE WATER LEVELS IN THE DEWATERING WELLS, OBSERVATION WELLS AND STANDPIPES SHALL BE MEASURED AT THE FOLLOWING INTERVALS:

TIME FROM COMMENCEMENT OF PUMPING TEST (mins)	INTERVAL BETWEEN READINGS (mins)
0-30	5
30-60	10
60-120	15
120-360	30
360-END OF TEST	60

DURING THE RECOVERY PHASE, THE READINGS SHALL BE TAKEN CONTINUOUSLY UNTIL THE WATER LEVEL IN ALL OBSERVATION WELLS AND STANDPIPES HAVE RECOVERED TO THEIR PRE-TEST LEVELS OR FOR A PERIOD OF TWO DAYS, WHICHEVER IS THE SOONER. PRIOR TO TERMINATING READINGS, THE ARCHITECT SHALL BE NOTIFIED.

# (III) MONITORING OF CHECKPOINTS

1. DURING THIS TEST AND UNTIL ALL STANDPIPES/ OBSERVATION WELLS HAVE RECOVERED TO THEIR PRE-TEST LEVELS, ALL SETTLEMENT CHECKPOINTS TILTING CHECK POINTS AND UTILITY CHECK POINTS AS SHOWN ON THE DRAWING NO. ELS-01 SHALL BE MONITORIED ONCE PER DAY. THE RESULTS SHALL BE PRODUCED IN ACCORDANCE WITH NOTE (I) 13.

# (IV) PUMP TEST CRITERIA

- THE PUMPING TEST SHALL BE CONSIDERED ACCEPTABLE IF THE FOLLOWING CRITERIA ARE MET WHEN THE DESIGNATED WATER LEVEL IS ACHIEVED INSIDE THE SITE:
- a. NO UNDUE SETTLEMENT OR MOVEMENT OF ANY SETTLEMENT CHECKPOINTS OR TILTING
- CHECKPOINTS AS STATED IN APPROVED EXCAVATION AND LATERAL SUPPORT WORKS PLAN OR NO
- DEFECT/DAMAGE TO ADJACENT GROUND/ STRUCTURES/ UTILITIES. b. THE GROUND SETTLEMENT DURING DEWATERING SHOULD NOT EXCEED 5.0mm.

# (V) ASSESSMENT REPORT

1. AFTER COMPLETION OF THE PUMPING TEST, THE CONTRACTOR SHALL PREPARE AN ASSESSMENT REPORT BASED ON THE TEST RESULTS DISCUSSING THE ASSUMED AND ACTUAL CONDITIONS ON SITE. INTERPERT THE RESULTS AND ASSESS THE EFFECTS TO THE SURROUNDING STRUCTURES AND UTILITIES. THIS REPORT SHALL BE SUBMITTED TO THE AP/RSE/RGE FOR VERIFICATION OF THE WATER CUT-OFF EFFECTIVENESS OF THE SHEET PILE WALL. THIS REPORT SHALL BE SUBMITTED TO BD'S SATISFACTION AFTER REVIEWED AND APPROVED BY AP/RSE/RGE.

# (VI) CONTINGENCY MEASURES

1. 2 NUMBERS OF RECHARGE WELL WOULD BE PROVIDED AS CONTINGENCY MEASURES IF GROUNDWATER DRAWDOWN EXCEEDING THE LIMIT AND UNSATISFACTORY PERFORMANCE DURING RECOVERY PHASE WERE FOUND. THE LOCATION OF RECHARGE WELL ARE SHOWN AT DWG. NO. ELS-13

#### FOR REFERENCE ONLY GENERAL NOTES ON PUMPING TEST

- 1. THE PUMPING WELLS SHOWN ARE MINIMUM REQUIREMENT ONLY. NOTWITHSTANDING THESE MINIMUM REQUIREMENTS, IT IS THE CONTRACTOR'S RESPONSIBILITY TO TAKE WHATEVER ADDITIONAL MEASURES THAT ARE NECESSARY TO ENSURE THE WATER LEVEL INSIDE THE SITE CAN BE LOWERED TO THE TO THE DESIGNATED LEVEL WITHOUT EXCEEDING THE DRAWDOWN AND SETTLEMENT CRITERIA STATED IN THIS DRAWING.
- 2. INSTALLATION RECORDS AND RESPONSE TEST RESULTS OF THE PIEZOMETERS, STANDPIPES, PUMPING WELLS AND OBSERVATION WELLS SHALL BE SUBMITTED PRIOR TO THE COMMENCEMENT OF THE PUMPING TEST.
- 3. THE PUMPING WELLS AND OBSERVATION WELLS FORM PART OF THE DEWATERING SYSTEM FOR THE FURTURE EXCAVATION.
- 4. THE TARGET GROUNDWATER TABLES TO BE LOWERED WITHIN THE SITE, AS RECORD BY OBSERVATION WELLS

			Т	ARGET			
OBSERVATION WELL	P1	P2	P3	P4	P5	P6	P7
DRAWDOWN LEVEL (mPD)	-8.075	-8.075	-8.075	-7.375	-10.325	-8.075	-8.075

- 5. THE PUMPING TEST SHALL BE STOPPED IN CASE THE GROUNDWATER DRAWDOWN OUTSIDE THE SITE EXCEEDS 2.0m. THE CONTRACTOR SHALL INVESTIGATE THE CAUSE OF THE DRAWDOWN AND IMPROVEMENT MEASURES SHALL BE PROPOSED AND IMPLEMENTED. CONTINGENCY MEASURES SUCH
- AS INSTALLATION OF RECHARGE WELLS BEHIND SHEET PILE WALLS MAY BE REQUIRED. 6. COMPLETE PUMPING TESTS RESULT SHALL BE SUBMITTED TO BD AFTER THE SUCCESSFUL COMPLETION.
- 7. PUMP WELLS AND OBSERVATION WELLS SHALL BE PROTECTED FROM DAMAGE. WORKS SHALL BE
- CARRIED OUT WITH DUE CARE IN PROXIMITY OF THOSE WELLS. 8. IN CASE THE PUMP/OBSERVATION WELL HAS BEEN DAMAGED DURING ANY TIME OF THE CONSTRUCTION WORKS, THE CONTRACTOR SHALL INFORM AP/RSE/RGE IMMEDIATELY AND REINSTATEMENT SHALL BE CARRIED OUT WITHOUT DELAY.

# NOTES FOR PARTIAL PUMPING TEST

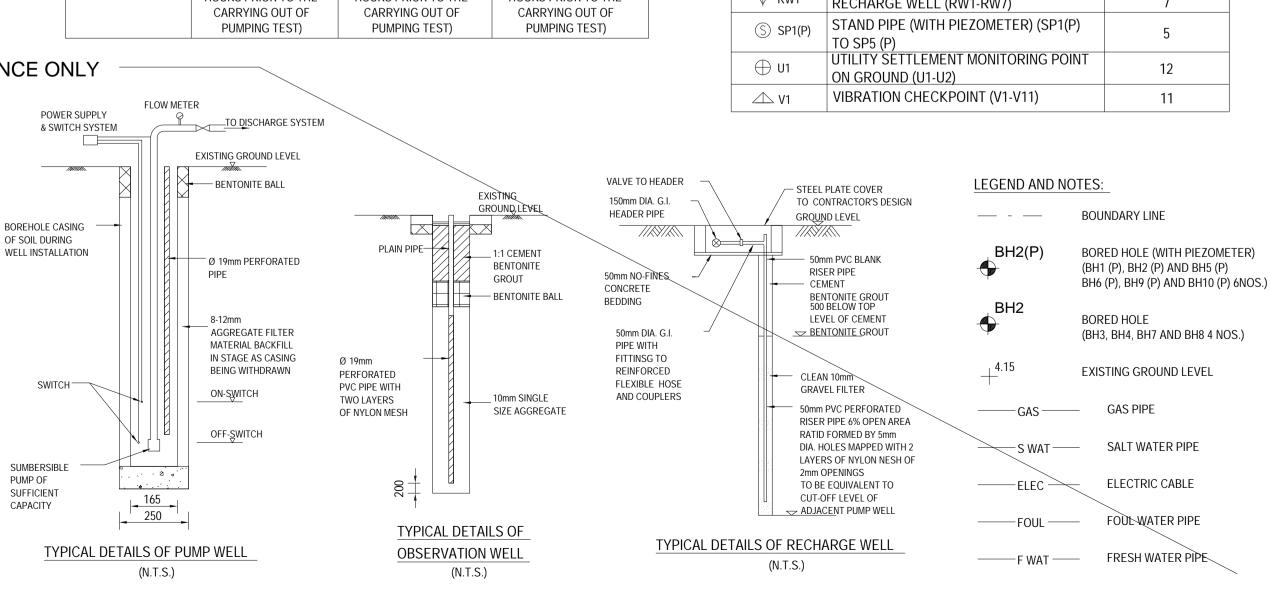
- 1. PARTIAL PUMPING TEST SHALL BE CARRIED OUT AFTER INSTALLATION OF PILES AND BEFORE
- COMMENCEMENT OF BULK EXCAVATION. 2. INSTALL PUMP WELL, OBSERVATION WELL, RECHARGING WELL AND PIEZOMETER.
- 3. THE PUMPING TEST PROPOSAL TO BE SUBMITTED SEPARATELY.
- 4. THE CRITERIA OF PUMPING TEST REFERS TO THE DWG. NO. ELS-13. 5. AFTER COMPLETION OF THE PARTIAL PUMPING TEST, AN ASSESSMENT REPORT SHALL BE PREPARED
- BY CONTRACTOR. THIS REPORT SHOULD BE SUBMITTED TO BUILDING AUTHORITY.

# MEASURED GROUNDWATER DRAWDOWN

		ALERT LEVEL	ALARM LEVEL	ACTION LEVEL
ODCEDVATIONIME	-116	0.75m BELOW THE	0.80m BELOW THE	0.87m BELOW THE
0002	OBSERVATION WELLS (OW2, OW4, OW6, OW6, OW8, OW10, OW12, OW14)	LOWEST MEASURED	LOWEST MEASURED	LOWEST MEASURED
		GROUND WATER TABLE	GROUND WATER TABLE	GROUND WATER TABLE
		(RECORDS WITHIN 72	(RECORDS WITHIN 72	(RECORDS WITHIN 72
OVV 12, OVV 14)		HOURS PRIOR TO THE	HOURS PRIOR TO THE	HOURS PRIOR TO THE
		CARRYING OUT OF	CARRYING OUT OF	CARRYING OUT OF
		PUMPING TEST)	PUMPING TEST)	PUMPING TEST)

ELS PUMPING TEST SETTING OUT PLAN

	INSTRUMENT SCHEDULE	
SYMBOL	TYPE	NUMBER
<b>♦</b> BS1	BUILDING SETTLEMENT MARKER (BS1-BS12)	12
<b>△</b> T1	11	
<b>S</b> 1	GROUND SETTLEMENT CHECK PONT (S1-S10)  O OW1 OBSERVATION WELL (OW1-OW14)  P P1 PUMP WELL (P1 TO P7)	
OW1		
P P1		
₩ RW1	RECHARGE WELL (RW1-RW7)	7
S SP1(P)	STAND PIPE (WITH PIEZOMETER) (SP1(P) TO SP5 (P)	5
<ul> <li>U1 UTILITY SÉTTLEMENT MONITORING POINT ON GROUND (U1-U2)</li> <li>✓ V1 VIBRATION CHECKPOINT (V1-V11)</li> </ul>		12
		11



CIC SAMPLE PROJECT DRAWING TITLE **EXCAVATION & LATERAL SUPPORT** WORKS PUMPING TEST SETTING OUT PLAN SCALE AS SHOWN@A1 DRAWING NO. SOURCE ---90mm (W) x 40mm (H) space for COMPANY LOGO

90mm (W) x 60mm (H) space for AP/RSE/RGE's signature/ and stamp chop

**BD's OFFICAL USE** 

90mm (W) x 150mm (H) space for BD's approval stamp / certification of copies of approved plans (PNAP ADM-10 APP A)